LIMA TOWNSHIP HALL EXPANSION

LIMA TOWNSHIP, WASHTENAW CO., MI PRELIMINARY SITE PLAN

OWNER/APPLICANT

TOWNSHIP OF LIMA
11452 JACKSON ROAD
DEXTER, MI, 48130
CONTACT: DUANE LUICK
734-475-2246

ENGINEER/SURVEYOR/LANDSCAPE ARCH.

MIDWESTERN CONSULTING, LLC 3815 PLAZA DR. ANN ARBOR, MI 48108 CONTACT: ADAM LALIK 734-995-0200

ARCHITECT

DANGEROUS ARCHITECTS
104 S MAIN STREET
CHELSEA, MI, 48118
CONTACT: SCOTT MCELRATH
734-475-3660

PROJECT NARRATIVE

LIMA TOWNSHIP HAS REQUESTED PREPARATION OF A PRELIMINARY SITE PLAN FOR THE EXPANSION OF THE TOWNSHIP BUILDING LOCATED AT 11452 JACKSON ROAD. THE EXPANSION CONSISTS OF A SINGLE FLOOR BUILDING EXPANSION AND A WALKOUT BASEMENT.

SITE IMPROVEMENTS INCLUDE 20 PAVED PARKING SPACES, ADA ACCESS AND RAMPS TO THE EXPANDED PORTION OF THE BUILDING. PROJECT INCLUDES A PAVED 2—WAY DRIVEWAY ENTRY WHICH WILL BE CONSTRUCTED AT THE EXISTING APPROVED DRIVEWAY LOCATION.

THE USE OF THE BUILDING EXPANSION WILL BE PRIMARILY FOR TOWNSHIP OFFICIAL OFFICES WITH APPROXIMATELY 6 FULL TIME EMPLOYEES, AND WILL NOT CAUSE A SIGNIFICANT INCREASE IN TRAFFIC GENERATION.

LEGAL DESCRIPTION

OLD SID - G 07-040-002-00 LI 37-2 LOT 11 EXC THE S 25 FT IN WIDTH THEREOF, ALSO ENTIRE LOT 4 BLK 1 VILLAGE OF HARFORD.

AND,

OLD SID — G 07-040-012-00 LI 37-12 LOT 10 EXCEPT S 25 FT BLK 1 VILLAGE OF HARFORD.

AND,

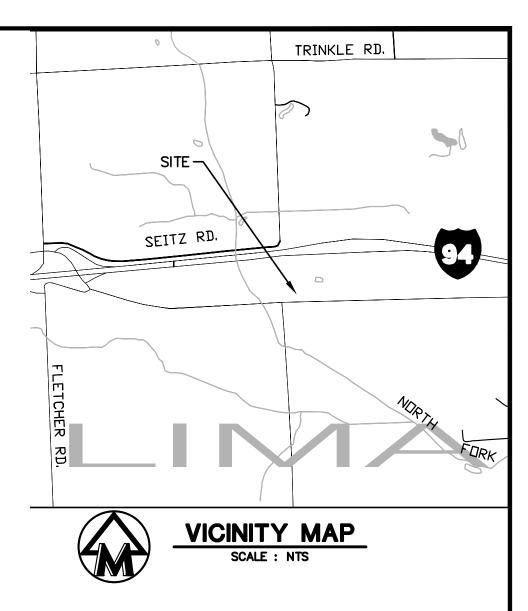
OLD SID - G 07-040-003-00 LI 37-3 LOTS 5,6 & 7 AND 12, 13 & 14EXC THE S 25 FT IN WIDTH OF LOTS 12, 13 & 14 BLK 1 VILLAGE OF HARFORD.

SITE DATA

SEE ARCHITECTURAL PLANS FOR APPLICABLE SITE DATA AND ZONING INFORMATION TABLE



SITE MAP



SHEET INDEX

SHEET TITLE

1 COVER SHEET

EXISTING CONDITIONS & REMOVALS

SITE PLAN

04 UTILITY PLAN 05 GRADING PLAN

DETAILED GRADING PLAN

O7 SITE DETAILS

LIMA TOWNSHIP HALL EXPANSION



M I D W E S T E R N

CONSULTING

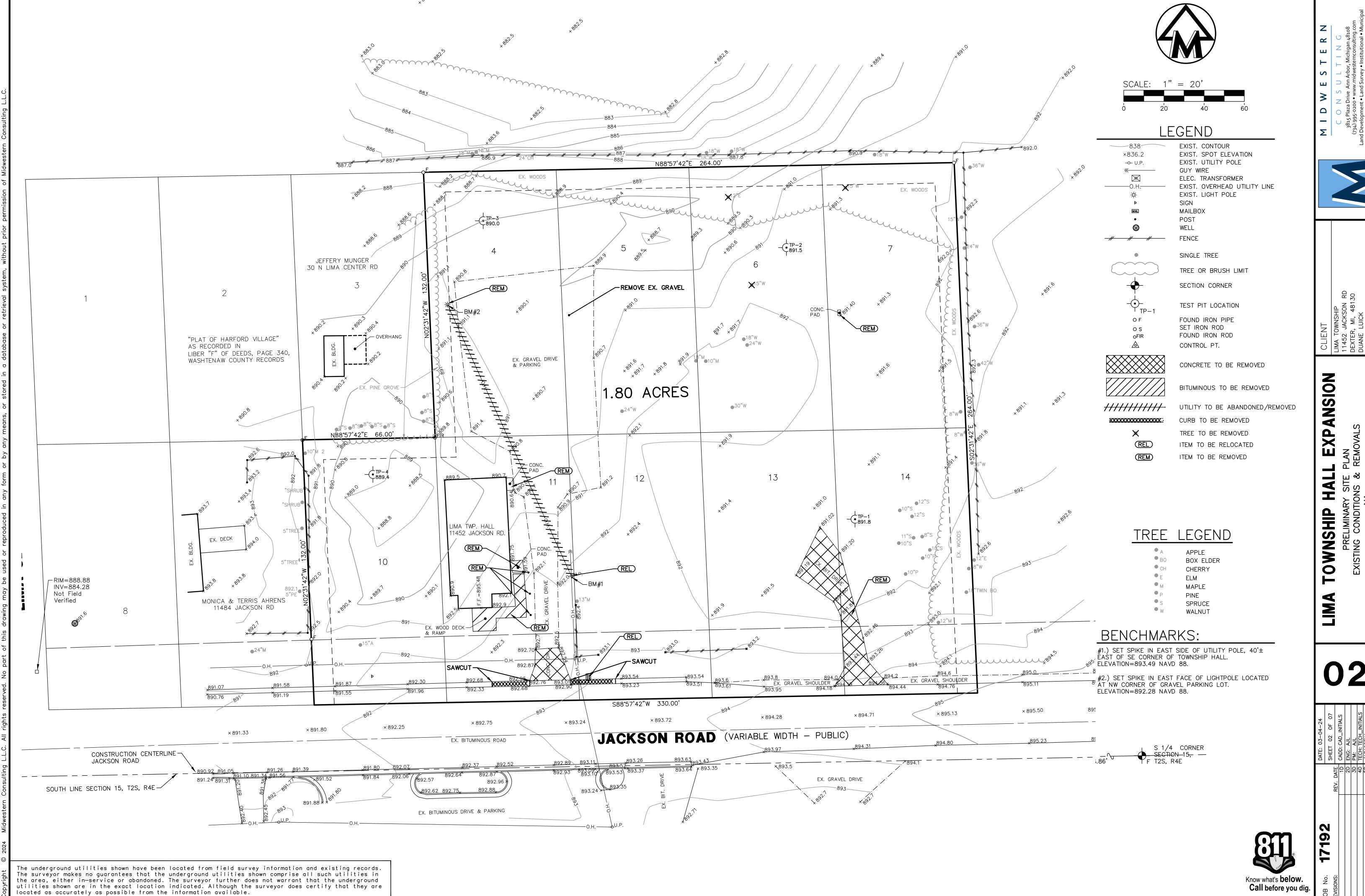
3815 Plaza Drive Ann Arbor, Michigan 48108
(734) 995-0200 • www.midwesternconsulting.com

Land Development • Land Survey • Institutional • Municipal
Wireless Communications • Transportation • Landfill Services

RELEASED FOR:	DATE	, all
		Mark S
		LICEN
		A SE

ADAM J
LALIK
ENGINEER
No.
6201309288

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Call before you dig.



NOTES

- 1 1/2" MDOT 4A BITUMINOUS MIX TOP COURSE

MAX. DENSITY. PLACE IN TWO LIFTS.

SUBBASE COMPACTED TO 95% MAX. DENSITY

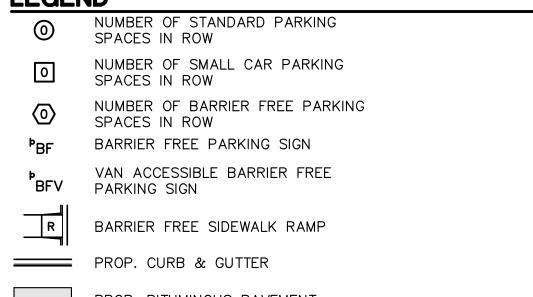
BITUMINOUS PAVEMENT DETAIL

-1 1/2" MDOT 3C BITUMINOUS MIX LEVELING COURSE

-8" MDOT 21AA CRUSHED LIMESTONE COMPACTED TO 95%

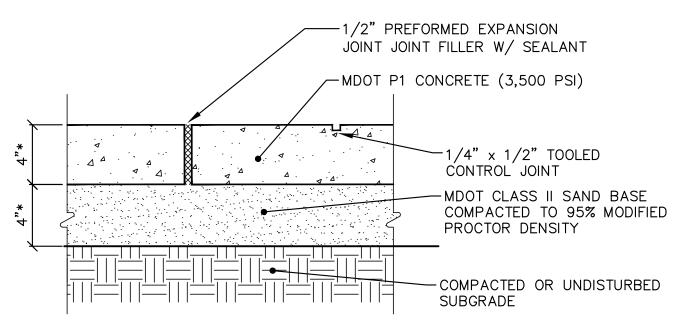
- 1. ALL DIMENSIONS ARE MEASURED TO THE PAINT LINE OR FACE OF CURB UNLESS OTHERWISE NOTED. ALL RADII DIMENSIONS SHOWN ARE TO BACK OF CURB UNLESS OTHERWISE NOTED.
- 2. ALL WORK SHALL BE COMPLETED IN ACCORDANCE WITH CURRENT STANDARDS, SPECIFICATIONS, AND GENERAL CONDITIONS OF THE AUTHORITY HAVING JURISDICTION.
- 3. REFER TO THE ARCHITECTURAL PLANS FOR DETAILS REGARDING THE SCOPE OF WORK FOR THE BUILDING ELEVATIONS, INTERIORS, AND APPURTENANCES.
- 4. THE CONTRACTOR SHALL CONTACT THE OWNER AND/OR ENGINEER PRIOR TO COMMENCING WORK SHOULD THERE BE ANY FIELD CONFLICTS WITH THE DESIGN INTENT.
- 5. NO ONSITE TRASH PICKUP. TRASH WILL BE TAKEN TO CURB FOR

LEGEND



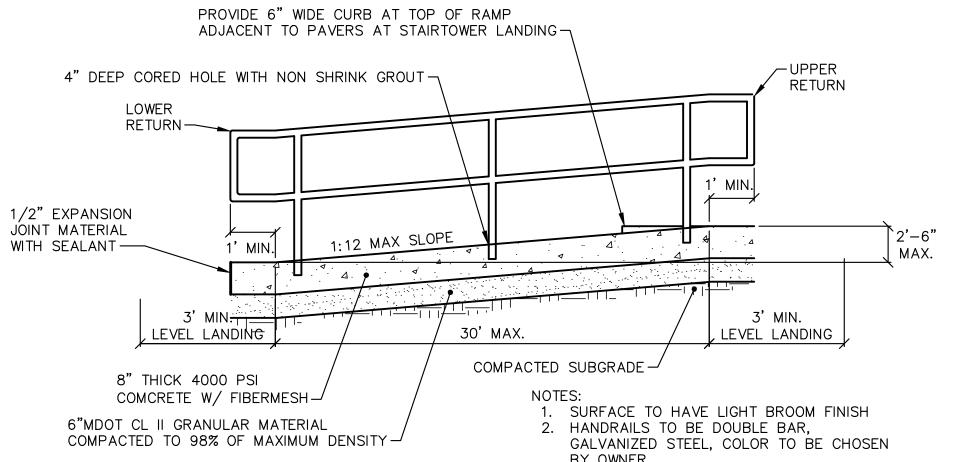
PROP. BITUMINOUS PAVEMENT PROP. CONCRETE PAVEMENT

SIGN PROP. SINGLE LIGHT PROP. DOUBLE LIGHT



* INCREASE CONCRETE WALK TO 6" WHEN CROSSING A SINGLE-FAMILY OR DOUBLE-FAMILY DRIVEWAY, AND TO 8" FOR COMMERCIAL DRIVE CROSSINGS. USE 6" CLASS II SAND BASE AT RESIDENTIAL DRIVE CROSSINGS, AND 8" 21AA AGGREGATE BASE (98% MODIFIED PROCTOR) AT COMMERCIAL DRIVE CROSSINGS.

CONCRETE WALK DETAIL



N88°57'42"E 264.00' , was a second with the second The state of the s PR. INFILTRATION BASIN JEFFERY MUNGER 50.79 30 N LIMA CENTER RD ~6" RAISED INTEGRAL WALK 10 SIDE YARD SETIBACK HARFORD VILLAGE" :DED IN OF DEEDS, PAGE 340, 10' SIDE YARD 1-AW COUNTY RECORDS 10″M SETBACK EX. PINE GROVE PROPOSED **●**30"W ADDITION FF=895.48 1,793SFT N88°57'42"E 66.00' FLUSH EDGE WALK-LIMA TWP. HALL 11452 JACKSON RD 11"S● ●⁸ EDGE WALK -−RELOCATĖ POWER POLE. SETBACK A & TERRIS AHRENS 484 JACKSON RD 100' WELL ISOLATION DIŞTANCE ⊢PR. STEPS TYPE M OPENIN@88°57'42"W 330.00'
MOUNTABLE CURB TYPE M OPENING 40"W MOUNTABLE CURB JACKSON ROAD (VARIABLE WIDTH - PUBLIC)

CONCRETE RAMP WITH RAILS DETAIL

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HALI Y SITE PLAN

OWNSHIP
PRELIMINAR
SITE

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W1	-	Determining Post-De Rational Method Var	iables		s, Curve Numbe	rs, and Run	off Coeffici	ents			Detent
		Cover Type Building/Pavement	Sc	il Type	Area 13,5	50	Area	31	Runoff Coeff. (C) 0.95	(C) x (Area) 0.30	
		Grass Total		В	15,1 28,6 Weighted	70	0.0 0.0 C)x(Area))/(A	36	0.25	0.09	
		NRCS Variables (Per Cover Type	vious) Soil Type		Area	, ,)x(Area))/(A	, 	0.58 Curve Number	(CN) x (Aroo)	\neg
		Grass Total	В		15,1 15,1	20	0.3 0.3	35	69	(CN) x (Area) 0.24 0.24	_ E
		NCRS Variables (Imp	ervious)		Weighted C				69	0.24	_
		Cover Type Building/Pavement	Soil Type		Area 13,5		Area 0.3		Curve Number 98	(CN) x (Area) 0.30	
		Total			13,5 Weighted C	50	0.:	31	98	0.30	
W2	_	W2 - First Flush Rund	off Calculations (Vff)	vvolgined of	(Carricon	1)/(/ ((00/)/ (/ (roa rotal)			
		Vff = 1" x 1/12" x 4356		- ·· ,	where A=	0.66	and	d where C= C	.58		•
		Vff = 1" x 1712" x 4356	60 sft/ac x	0.60	6 x	0.58	=	1,386 c	f		
W3		W3 - Pre-Developme 2 year / 24 hour storm		off Calculations	S (Vbf-pre)		P=	2.35 ir	1		
	B.	Pre-Development CN (Developed Area, Good	I Condition - Type	B Soils)			CN=	61			
		S = (1000 / CN) - 10 $Q = [(P-0.2S)^2] / [P+0]$).8S]				S= Q=	6.393 ir 0.154 ir			
		Total Site Area excludiv Vbf-pre = $Q \times (1/12) \times A$		'BMPs			Vbf-pre =	28,670 s 367 c			
W4		Pervious Cover Post-		ınkfull Runoff (Calculations (V	bf-per-post)					
	В.	2 year / 24 hour storm Pervious Cover CN Fro					P= CN=	2.35 ir 69			
	D.	S = (1000 / CN) - 10 $Q = [(P-0.2S)^2] / [P+0.2S)^2$	-				S= Q=	4.493 ir 0.354 ir	1		
		Pervious Cover Area from Vbf -per-post = $Q \times (1/12)$				Vbf	-per-post =	15,120 s 447 c			
W5		W5 - Impervious Cov 2 year / 24 hour storm		ment Bankfull	Runoff Calcula	tions (Vbf-in	ıp-post) P=	2.35 ir	1		
	В.	Impervious Cover CN F S = (1000 / CN) - 10					P= CN= S=	2.35 ii 98 0.204 ii			
	D.	$Q = [(P-0.2S)^2] / [P+0]$ Impervious Cover Area	<u>-</u>				Q=	2.122 ir 13,550 s	1		
		Vbf-imp-post = Q x (1/12				Vbf	-imp-post =	2,396 c			
W6		W6 - Pervious Cover 100 year / 24 hour stor	-	nt 100-Year Ru	unoff Calculatio	ons (V100-pe	r-post) P=	5.11 ir	1		
	В.	Pervious Cover CN Fro S = (1000 / CN) - 10					CN= S=	69 4.493 ir			
		$Q = [(P-0.2S)^2] / [P+0]$ Pervious Cover Area from					Q=	2.038 ir 15,120 s			
	F.	$V100$ -per-post = $Q \times (1/$	12) x Area			V100-	-per-post =	2,567 c	ft		
W7	Α.	W7 - Impervious Cov 2 year / 24 hour storm	event:	ment 100-Year	Runoff Calcula	tions (V100-	imp-post) P=	5.11 ir	1		
	C.	Impervious Cover CN F S = (1000 / CN) - 10					CN= S=	98 0.204 ir			
	E.	$Q = [(P-0.2S)^2] / [P+0]$ Impervious Cover Area	from Worksheet 1			16.	Q=	4.873 ir 13,550 s	ft		
W8		Vbf-imp-post = $Q \times (1/12)$ Time of Concentration				Vbf	-imp-post =	5,502 c	π		
***		Assume 15-minute mir		centration	Tc=	0.25	hr				
W9	- A.	Runoff Summary & C Summary from Previou		n Requiremen	t						
		First Flush Volume (\ Pre-Development Ba	/ff)	lume (Vbf-pre)	1			1,386 c 367 c			
		Pervious Cover Post-D Impervious Cover Post	-Development Bai					447 c 2,396 c			
		Total BF Volume (Vb Pervious Cover Post-D	evelopment 100-Y	•				2,842 c 2,567 c	ft		
		Impervious Cover Post Total 100-Year Volun	ne (V100)		(V100-imp-post)			5,502 c 8,070 c			-11
	B.	Determine Onsite Infilte Subtract the Pre-Devel	opment Bankfull fi	om the Post-De	evelopment Bank	full Volume		0.040			- **
		Total Post-Development Bank	full Runoff Volume					2,842 c 367 c	ft		34 ?DS
		Bankfull Volume Diff Infiltration Requirem						2,475 c 2,475 c			
W10	- A.	Detention/Retention Qp = 238.6 Tc^-0.82	Requirement					743 63 6	fs/(in x sq. mi)		
	В.	Total Site Area excludi Q100 = Q100-per + Q10		'BMPs				0.66 a 6.911 ir	С		
	D.	(from W6 and W7, resp Peak Flow (PF) = Qp >	oectively) : Q100 x Area / 64	0				5.28 c			_
	E.	Delta = PF - 0.15 x Area [0.15 x Area (ac)]	ea (ac)					5.19 c 0.10 c	fs fs		
	F.	Vdet = Delta / PF x V1 Required Detention not	including infiltrati		alty.			7,919 c			
	_	Sediment Forebay Vol	ume Required (5%	of V100)				403 c	ft		
		ention Vret = 2 x V100						16,140 c	ft		//
W11	-	Determine Applicabl	e BMPs and Ass	ociated Volum	e Credits						
		Proposed BMP		Area (sft)	Storage Vo	lume (cft)	Design Ir	I .	Infilt. Volume in 6-hr Drawdown (cft)	Total Volume Reduction (cft)	
		Infiltration Bed		2678			1.	50	2,009 Structural BMPs (cft	2,009	91
					. 5.61. V			n Requireme	nt (Vinf) from W9 (cft) off Volume Credit (cft)	2,475	
W13		Site Summary of Infi		ion					(-1)		!
		Stormwater Manage Min Infiltration Requires	ment Summary ment (Vinf)					2,475 c			
		Designed/Provided Infil % Minimum Require	d Infiltration Provid					2,009 c	, 0		5
		Total Calculated Detention Net Required Detention	n Volume					7,919 c 5,911 c			
	_	(Vdet - Designed/Pro		·		_					
	B.	Detention Volume In % Required Infiltration	NOT Provided	-	ured infiltration	n volume ca	innot be ac	hieved. 18.8 %	ó		13
		(100% - % Minimum Net % Penalty (20% x	% Required Infiltr	ation NOT Provi	•			3.8 %			
		Total Required Deter			t y enalty) x Net Re	quired Deten	tion Volume)	8,218 c	ıı		
											— —

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utilities shown are in the exact location indicated. Although the surveyor does certify that they are located as accurately as possible from the information available.

Detention Ou	utlet Calculcations					
_ A. Re	quired Detention Volumes (Reduced by	6-hour infiltration	on)			
Sto	orm Event	Req'd Volume	less	Infil. Credit	=	Final Volume
Fire	st Flush	1,386 cft	-	2,009 cft	=	(623) cft

Storm Event	Req'd Volume	less	Infil. Credit	=	Final Volume
First Flush	1,386 cft	-	2,009 cft	=	(623) cft
Bankfull	2,842 cft	-	2,009 cft	=	834 cft
100 -year	7,919 cft	-	2,009 cft	=	5,911 cft
100-year + Req'd Penalty	8,218 cft	-	2,009 cft	=	6,209 cft
Forebay Volume Required (5% of 100-y	r)			=	296 cft

tention Volumes Provided

Elevation	Area (sft)	Depth (ft)	Volume (cft)	Cum. Volume (cft)
887.0	2,678	0	0	0
888.0	3,401	1	3,032	3,032
889.0	4,180	1	3,784	6,816
			Total Volume =	6,816

I Infiltration Design

Time to Fully Drain	24.5 hr
Infiltration Flow Rate	334.75 cft/h
Infiltration Rate, Average	1.50 in/hr
Infiltration Area	2678 sft
Total Storage Volume	8,218 cft

iis is less than 48 hours max, so the basin complies with the drawdown requirement.

JEFFERY MUNGER

30 N LIMA CENTER RD

N88°57'42"E 66.00'

PR.\ WELL LOCATION. -

× 892.25

× 891.80

LIMA TWP. HALL 11452 JACKSON RE

× 892.75

EX. WOODS

NARRATIVE AND NOTES

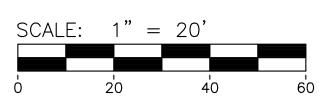
- 1. THE STORMWATER ON SITE WILL BE COLLECTED WITHIN TWO CATCH BASINS ON SITE. THE STORM WATER CONVEYANCE SYSTEM WILL DISCHARGE STORM RUNOFF TO THE PROPOSED INFILTRATION BASIN ON THE NORTH EAST PORTION OF THE SITE. RUNOFF COLLECTED WILL BE INFILTRATED INTO THE SOIL AND DRAW DOWN WITHIN 24.5 HOURS.
- 2. STORMWATER PIPE TO BE REINFORCED CONCRETE PIPE (RCP)
- 4. CB CASTINGS SHALL BE EJIW 1040 W M1 FLAT COVER

N88°57'42"E 264.00'

12" NE INV=887.34 12" S INV=887.47

- 5. SEPTIC/DRAINFIELD DESIGN TO BE SUBMITTED UNDER A SEPARATE SUBMITTAL TO THÉ WASHTENAW COUNTY ENVIRONMENTAL HEALTH DEPARTMENT.
- 6. ALL NECESSARY PERMITS SHALL BE ACQUIRED THROUGH STATE AND LOCAL AGENCIES FOR THE PROPOSED REPLACEMENT WELL.











EXIST. SPOT ELEVATION PROP. SPOT ELEVATION EXIST. UTILITY POLE		
EXIST. UTILITY POLE W/ TRANS.		
EXIST. GUY POLE		
GUY WIRE		
ELEC. TRANSFORMER		
EXIST. AC UNIT		7 2 6
EXIST. GENERATOR		IP SON 481
EXIST. OVERHEAD UTILITY LINE		[호 옷 특
EXIST. LIGHT POLE	<u> </u>	§ ĕ Š ²
PROP. LIGHT POLE		[2 년 1년 기 년

CLIENI LIMA TOWNSHIP 11452 JACKSON F DEXTER, MI, 4813 DUANE LUICK
734-475-2246

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	DAIE: 03-04-24
	SHEET 04 OF 07
tev. DATE	1D CADD: CAD_INITIALS
2D	ENG: AJL
3D	PM: AJL
4D	TECH: TECH_INITIALS
2D	/17192SW1
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L L	KEV. DAIE	1D	2D	3D	4D	2D	(PD

EXIST. SOIL BORING LOCATION EXIST. TEST PIT LOCATION

TREE OR BRUSH LIMIT

SECTION CORNER

JACKSON ROAD

-RELOCATE UTILITY POLE

S88°57'42"W 330.00'

RIM = 892.53

12" N INV=888.00

3. CATCH BASIN STRUCTURES ARE TO BE 4' DIA. PRECAST CONCRETE

- SEDIMENT

12" SW INV=887.00

1.80 ACRES

POTENTIAL DRAINFIELD

LOCATION

× 894.28

●10"S

× 895.13

-CTP-1 891.8

× 894.71

Call before you dig.

EXIST. CONTOUR PROP. CONTOUR

PROP. BUILDING LIGHT EXIST. TELEPHONE LINE PROP. TELEPHONE LINE EXIST. ELECTRIC LINE PROP. ELECTRIC LINE EXIST. GAS LINE

PROP. GAS LINE EXIST. GAS VALVE

EXIST. FIBER OPTIC LINE

PROP. FIBER OPTIC LINE

EXIST. GATE VALVE IN BOX

PROP. GATE VALVE IN BOX EXIST. GATE VALVE IN WELL

PROP. GATE VALVE IN WELL EXIST. CURB STOP & BOX PROP. CURB STOP & BOX

EXIST. WATER MAIN

PROP. WATER MAIN

EXIST. HYDRANT PROP. HYDRANT

REDUCER

EXIST. BLOW-OFF

PROP. BLOW-OFF

THRUST BLOCK

PROP. KNOXBOX

EXIST. STORM SEWER PROP. STORM SEWER

EXIST. BEEHIVE INLET PROP. BEEHIVE INLET PROP. ROOF DRAIN

EXIST. DOWNSPOUT PROP. DOWNSPOUT

PROP. SANITARY SEWER

DRAINAGE DIRECTION CONCRETE SURFACE ENCLOSED TRASH AREA

RAILROAD CROSSING SIGN EXIST. PARKING METER PROP. PARKING METER

END SECTION HEAD WALL

C/L OF DITCH SIDEWALK RAMP

MAILBOX

GUARDRAIL SINGLE TREE

WELL

CULVERT

----s--O--- EXIST. SANITARY SEWER

----S------

 $\Rightarrow \Rightarrow$

-**1** TP−1

EXIST. CATCH BASIN OR INLET

PROP. CATCH BASIN OR INLET

POST INDICATOR VALVE

POST INDICATOR VALVE

EXIST. FIRE DEPARTMENT CONNECTION PROP. FIRE DEPARTMENT CONNECTION

LEGEND

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NG PLAN

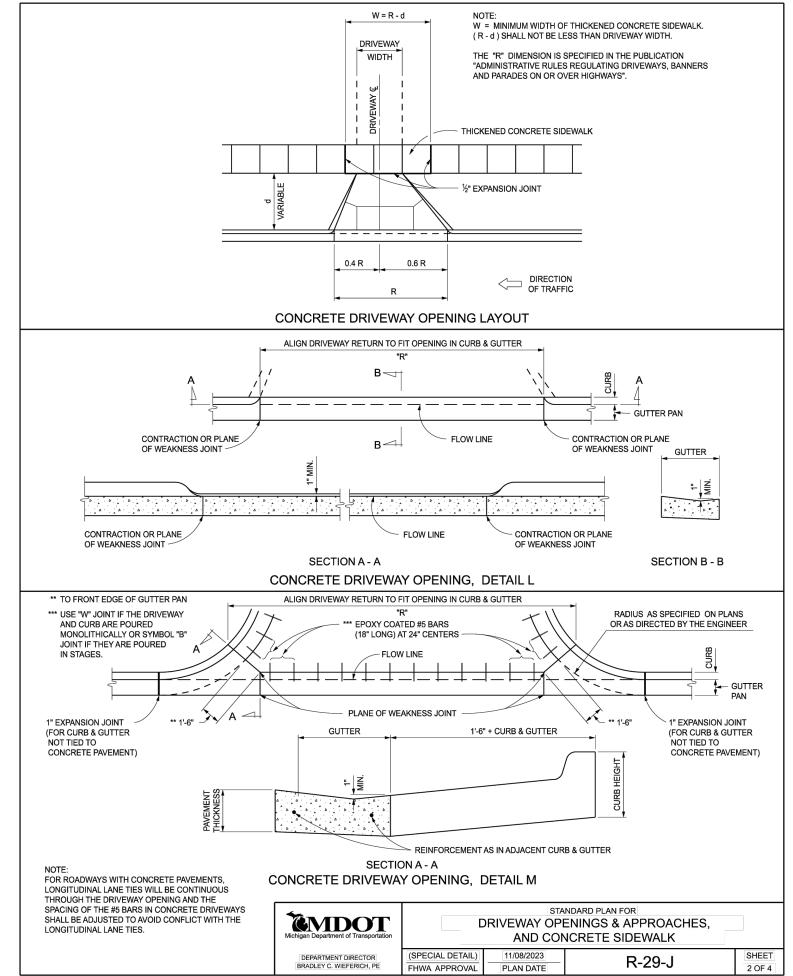
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PRELIMINAR
GRADIN

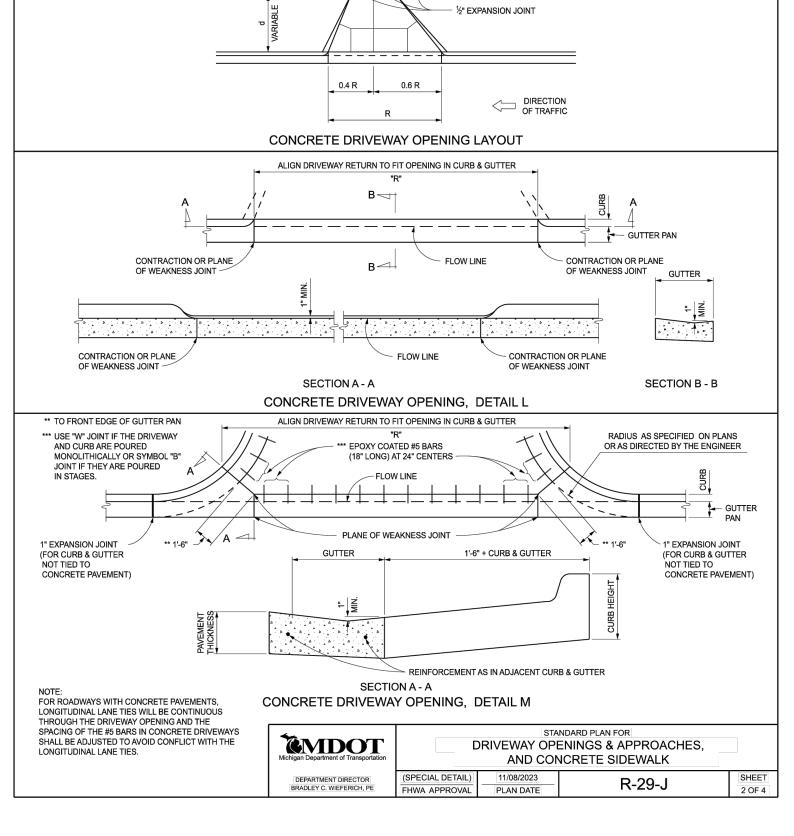
REVISIONS:

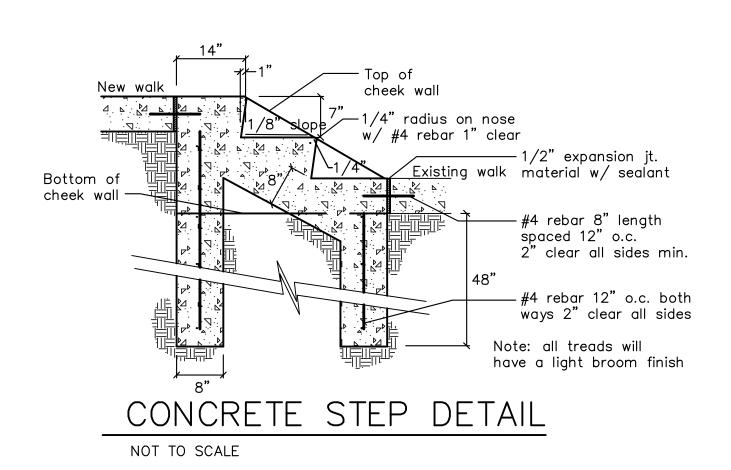


DATE: 03-04-24	SHEET 06 OF 07	SINITINI CAD: CAD	טלט. טלטרט	ENG: AJL	PM: AJL	TECH: TECH_INITIALS	/17192GP1	FB#
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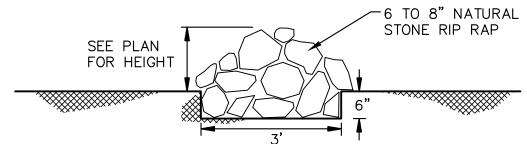
TYPICAL INTEGRAL WALK & CURB NOT TO SCALE

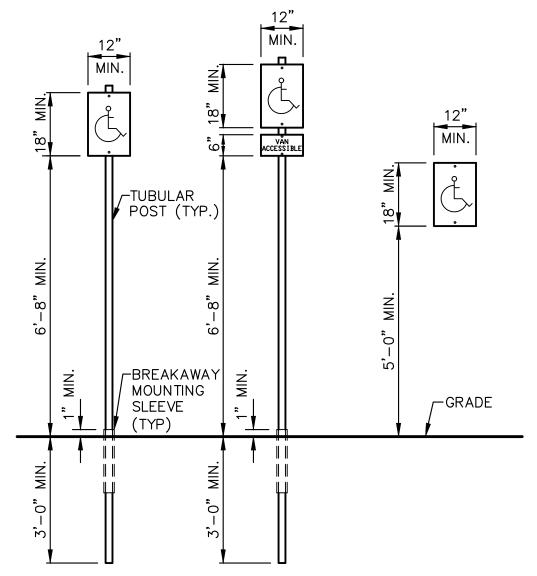






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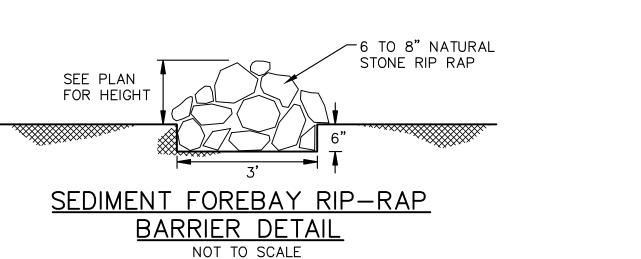


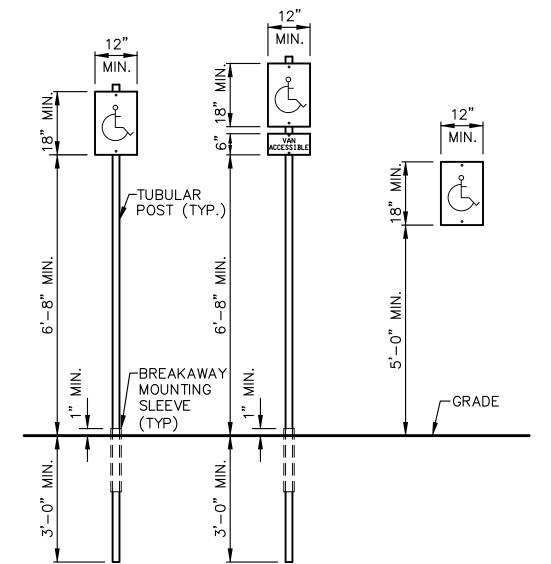
TYPICAL HANDICAP PARKING SIGNS

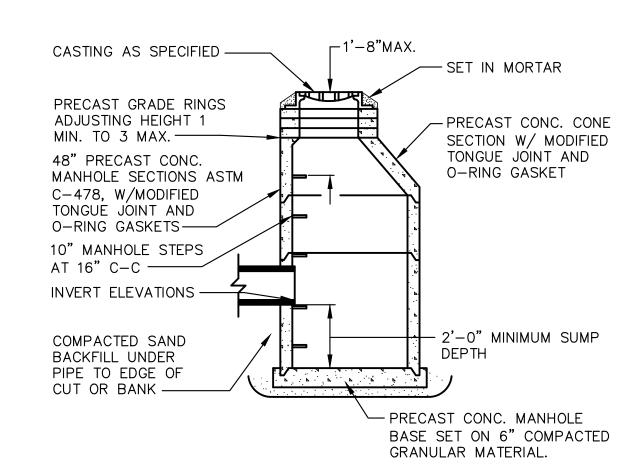
SCALE: 1/2" = 1'-0"

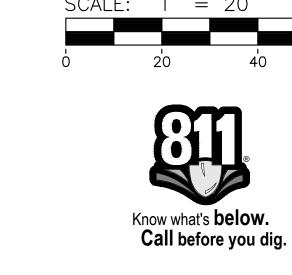
WALL MOUNTED SIGNS

POST MOUNTED SIGNS



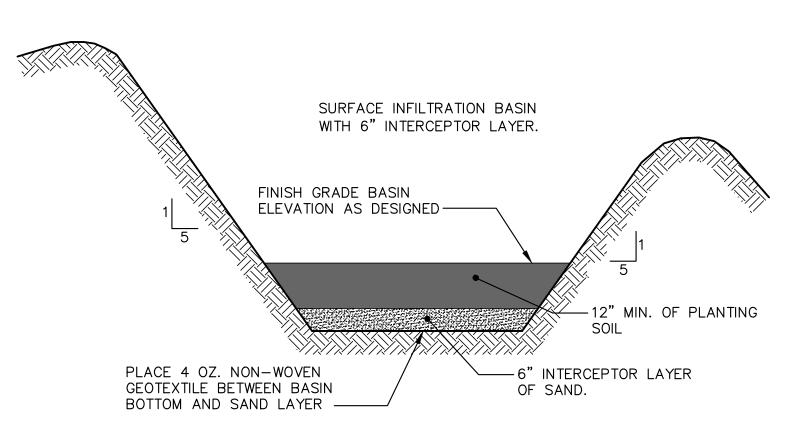






4' DIA. CATCH BASIN

NOT TO SCALE

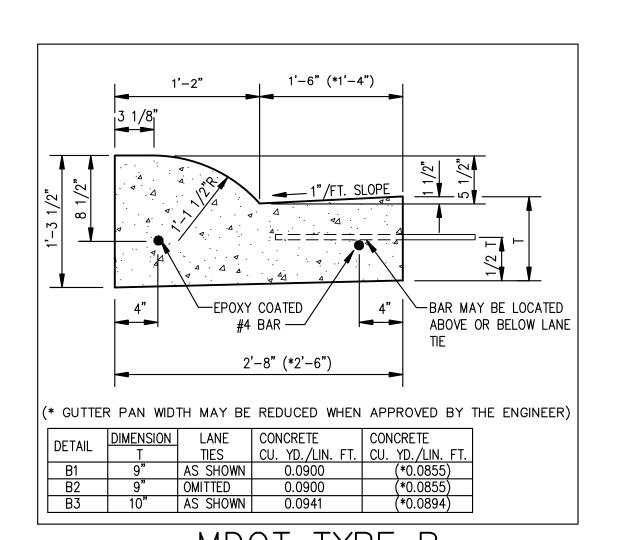


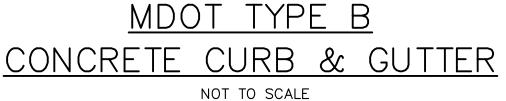
NOTES:

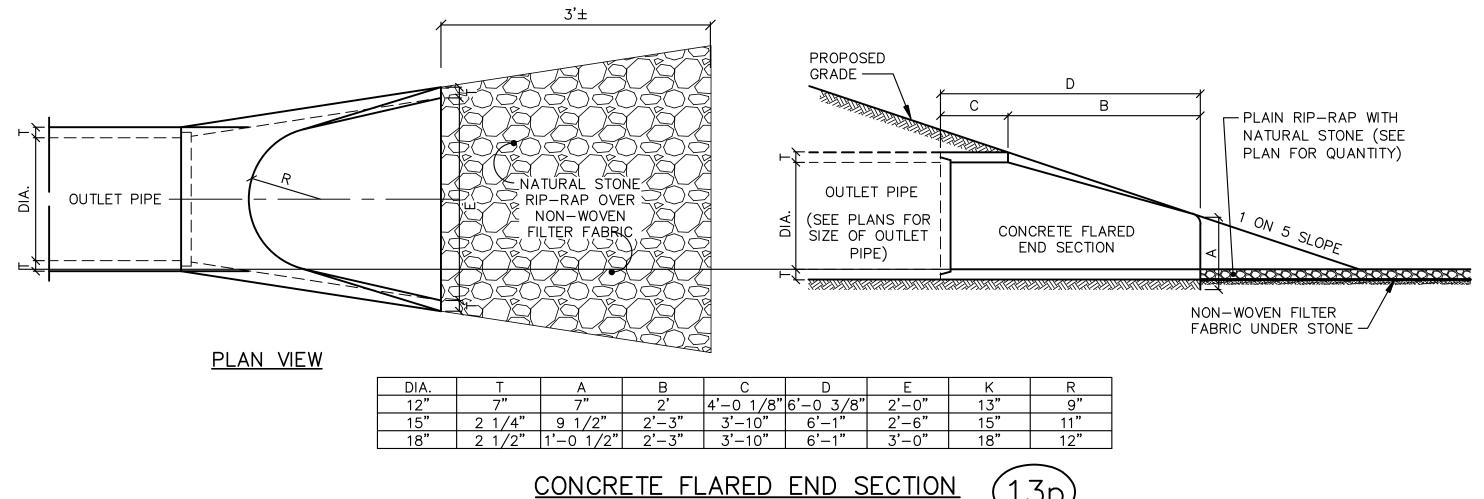
1. THE SAND LAYER AND GEOTEXTILE SHALL NOT BE PLACED IN THE BOTTOM OF THE INFILTRATION BASIN UNTIL ONSITE SOIL STABILIZATION IS IMMINENT TO PREVENT THE SILTING OF THE SAND LAYER DURING THE CONSTRUCTION PERIOD.

2. THE BOTTOM OF THE POND SHALL BE EXCAVATED TO A POINT 6 INCHES HIGHER THAN THE SUB BASE GRADE AND SHALL REMAIN AT THAT ELEVATION UNTIL ONSITE SOIL STABILIZATION IS IMMINENT AND THE SAND LAYER IS PLANNED TO BE INSTALLED. AT THAT TIME THE FINAL 6 INCHES OF MATERIAL SHALL BE REMOVED, ALONG WITH ANY SILTS THAT HAVE ACCUMULATED, LEAVING A CLEAN, SANDY BOTTOM. THE ENGINEER SHALL BE CONTACTED FOR INSPECTION PRIOR TO GEOTEXTILE AND SAND LAYER PLACEMENT.

INFILTRATION BASIN WITH INTERCEPTOR LAYER CROSS SECTION







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OWNSHIP
PRELIMINAR
SITE